NONLINEAR FRACTURE PROPERTIES FROM SIZE EFFECT TESTS

By Zdeněk P. Bažant,¹ F. ASCE, Jin-Keun Kim,¹ and Phillip A. Pfeiffer²

ABSTRACT: The previously derived size effect law for blunt fracture is exploited for determining the parameters of the R-curve, of the crack band model, and of Hillerborg's fictitious crack model. No measurements of the crack length or of the unloading compliance are needed. It suffices to measure only the maximum load values for a set of geometrically similar specimens of different sizes. The parameters of the size effect law can then be identified by linear regression. The inverse slope of the regression line yields the fracture energy. The regression also has a twofold benefit: it smooths statistically scattered data, and it extends the range of the data, so that one can do with fewer tests. From the experimentally calibrated size effect law, the R-curve may then be obtained as the envelope of the family of fracture equilibrium curves for different specimen sizes. A simple algebraic formula for this envelope is presented. The size effect regression plot makes it also possible to determine crack band model parameters, particularly the fracture energy, the crack band width, and the strain-softening modulus. The same is made possible for Hillerborg's model.

INTRODUCTION

The fracture properties of the materials in which the crack front is blunted by microcracking or yielding are not completely described by a single parameter, the fracture energy. At least two further parameters, e.g., the strength and a characteristic length, are required. Several mathematical models with additional parameters have been recently formulated (3,4,7,10,13,18,23) and shown capable of closely representing the available experimental evidence. Determination of their fracture parameters, however, is a problem. This problem will be addressed in what follows.

The simplest, although crudest, method consists of an approximate linearly elastic fracture analysis using some equivalent crack length and the R-curve, describing how the energy resistance to crack growth depends on the crack length. Irwin and Krafft et al. (14,16) proposed that the R-curve may be considered to be approximately unique. Shah and coworkers (21,22) introduced the R-curve concept to concrete. The R-curve is usually determined on a single specimen from the crack lengths observed at various load values, or on a series of specimens from their critical load values and the corresponding critical crack lengths. However, the need to measure the crack length is a considerable obstacle for a material like concrete. The crack length is hard to define since the crack tip is blurred by a microcracking zone, and even if one succeeds to locate the crack tip, its meaning is of dubious significance since the R-curve is actually a function of a certain equivalent crack length which yields the correct remote elastic stress field, rather than of the actual crack length. To avoid these difficulties, it has been attempted to determine the crack length indirectly, by measuring the specimen's compliance, either at unloading or at reloading. But this is also questionable because at unloading or reloading the microcracks within the fracture process zone do not completely cease, causing the compliance for unloading and reloading to be smaller than the compliance for continued loading, which, however, cannot be measured since the crack growth cannot be arrested.

The R-curve is a device to make possible an approximate linearly elastic solution (7). A nonlinear finite element solution, which is more realistic, may be obtained with the blunt crack band model (3,9,10) in which a suitable triaxial tensile strain-softening constitutive relation is used and the crack band front is assumed to have a certain characteristic width that is a material property. Equivalent results can be obtained with Hillerborg's model (13,18), based on a softening stress-displacement relation. Following an idea suggested in Ref. 3, it will be shown that the material parameters for these models may be easily determined from the size effect.

SIZE EFFECT LAW

For blunt fracture one can generally introduce the hypothesis (5,9) that the total potential energy release W caused by fracture in a given structure depends on both: (1) The length a of the fracture; and (2) the area traversed by the fracture process zone, such that the size of the fracture process zone at failure is constant, independent of the size of the structure. Dimensional analysis and similitude arguments then show that, for geometrically similar specimens or structures made of the same material (5,6)

$$
\sigma_a = B f^2 \left(1 + \frac{d}{d_0}\right)^{-1/2}
$$

in which $\sigma_a = P / bd$ = nominal stress at maximum load; $P$ = thickness; $d = \gamma$ characteristic dimension of the structure (e.g., beam's depth); $f^2 = \gamma$ direct tensile strength; $d_0 = \gamma_0 d_s; d_s = \gamma$ aggregate size; and $B, \gamma_0, d_s = \gamma$ empirical constants. See the graph in Fig. 1. The values of $B$ and $\gamma_0$ depend on the geometrical shape of the structure.

If the structure is very small, then $d / d_0 \ll 1$, and so $\sigma_a \approx B f^2$. This is the strength (or yield) criterion, represented by a horizontal line in Fig. 1(a). If the structure is very large, then $d / d_0 >> 1$, and so $\sigma_a \sim \gamma_0$. This is the size effect typical of linear elastic fracture mechanics which corresponds to the inclined straight line in Fig. 1(a), having the slope $-1/2$. The size effect law according to Eq. 1 represents a gradual transition from the strength (or yield) criterion to the energy criterion of linear elastic fracture mechanics. This law is only approxi-
mate because the hypothesis of a constant size of the fracture process zone at failure is not exact.

**Equivalent Linear Fracture Analysis and R-Curves**

Frequently, the fracture process zone is small compared to the structure dimensions and the remote stress and strain fields are almost the same as the elastic ones. The fracture may then be approximated treated as an equivalent line crack that produces the same remote elastic stress and strain fields. For this equivalent crack, the energy, \( R \), required for crack growth (also called the fracture resistance) must be considered as a function of the crack extension \( c \) from the notch or smooth surface, i.e., \( R = R(c) \) in which \( c = a - a_0 \); \( a_0 \) is length of the notch; and \( a = \) total length of the crack plus notch (Fig. 2). The energy that must be supplied to produce the crack is \( U = b f R(c) d a = W(a) \) where \( W(a) = \) the total release of strain energy from the structure. An equilibrium state of fracture occurs if \( \delta U = 0 \). Since \( \delta U = (\partial U/\partial a) \delta a = 0 \), in which \( \partial U/\partial a = b(R - G) = 0 \), and \( bG = W' = \partial W/\partial a \), it follows that fracture equilibrium takes place if \( G(a) = R(c) \), in which \( G \) is the energy release rate of the structure. The equilibrium fracture state is stable if \( \delta^2 U \) is positive definite. Since \( \delta^2 U = (\partial^2 U/\partial a^2) \delta a^2 \) where \( \delta^2 U/\partial a^2 = b[dR/dc - \partial G(a)/\partial a] = B[R'(c) - G'(a)] \) and \( bG'(a) = W''(a) = \delta^2 W/\partial a^2 \), the following conditions ensue:

\[
R'(c) - G'(a) > 0 \quad \text{stable}; \quad R'(c) - G'(a) = 0 \quad \text{critical} \quad \cdots \quad (2)
\]

Considering structures that are geometrically similar but could have dissimilar notches, and using dimensional analysis, one can show that

\[
G(a) = \frac{W'(a)}{b} = \frac{P^2 g(a)}{E_b b^2 d} \quad \cdots \quad (3a)
\]

\[
G'(a) = \frac{W''(a)}{b} = \frac{P^2 g'(a)}{E_b b^2 d^2} \quad \cdots \quad (3b)
\]

in which \( \alpha = a/d = (a_0 + c)/d \); \( g'(a) = d g(a)/d a \); \( E_b \) = Young's elastic modulus; and \( g(a) \) = nondimensional function which characterizes the geometry of the structure and can be determined by linear finite element analysis or found for typical shapes in handbooks (19) \( G = K_i/E_b \), \( K_i = \) stress intensity factor. Substitution of Eqs. 3a–b into Eq. 2 leads to the conditions

\[
R(c) g'(a) - R'(c) g(a) d > 0 \quad \text{stable};
\]

\[
R(c) g'(a) - R'(c) g(a) d = 0 \quad \text{critical} \quad \cdots \quad (4)
\]

Alternatively, since \( dg/dc = dg/da = g'(a)/d \), this is equivalent to \( R(c)/g(a) = \) max. Thus, one may simply try many values of \( c \) and identify which one yields the maximum.

The R-curve concept rests on the hypothesis (14,16) that function \( R(c) \) may be approximately considered to be a unique material property. Keep in mind, though, that function \( R(c) \) is generally not exactly the same for different specimen shapes and different notch lengths.

**Identification of Fracture Energy from Size Effect**

As an example, consider three-point bent fracture tests on specimens of various depths \( d \), all with thickness \( b = 1 \) in., span to depth ratio \( L/d = 4 \), and initial notch depth \( a_0 = d/5 \); also, \( E_b = 4.3 \times 10^6 \) psi and \( f_c = 3,650 \) psi (15). According to Tada (19), \( g(a) = 16 \pi \alpha (1.635 - 2.603 \alpha \)
+ 12.30 \alpha^2 - 21.27 \alpha^3 + 21.86 \alpha^4)^2. \text{ The following experimental data are considered:}
\begin{align*}
d & = 1.5, \quad 3^*, \quad 6^*, \quad 9^*, \quad 12, \quad 15, \quad 18, \quad 21 \text{ in.;} \\
P & = 82.4, 156.9^*, \text{ and } 167.4^*, 239.6^*, 295.6^*, 490.0, 514.5, 555.5, 700 \text{ lbs.} \tag{5}
\end{align*}
(asterisks label the values taken from Jenq and Shah’s measurements, Ref. 15). The inevitable statistical scatter may be smoothed out by applying the size effect law in Eq. 1. This equation may be algebraically transformed to a linear form, \( Y = AX + C \), in which \( X = d; Y = \alpha_N \sigma_N^2 = F d^2/P^2 \); \( A = C/d_0 \); and \( C = 1/(B f^2) \). Thus, coefficients \( A \) and \( C \) can be determined by linear regression, either by computer or by hand; see Fig. 1(b). This yields \( C = 2.742 \times 10^{-4} \) and \( A = 0.350 \times 10^{-4} \). The coefficient of variation \( \omega \) of the vertical deviations from the regression line and the corresponding 95% confidence limits are also shown in Fig. 1(b).

As mentioned before, in the limiting case of very large sizes (\( d/d_0 \gg 1 \)) linear elastic fracture mechanics applies. We call the corresponding limit value of \( R \) the fracture energy \( G_f \). It may be obtained from the inclined asymptote in Fig. 1(a), i.e., from Eq. 1 when \( L \) is neglected, in which case \( Y = AX = \alpha_N \sigma_N^2 \). Noting that \( \alpha \to \alpha_0 \) for failure of very large specimens, and substituting \( X = d \) and \( \sigma_N = G_f/E \sigma_0 d \), which follows from Eq. 3a by setting \( G = G_f \) and \( P = \sigma_N b d \), we obtain the result
\begin{equation}
G_f = \frac{g(\alpha_0)}{AE} \tag{6}
\end{equation}
in which \( \alpha_0 = \alpha_0/d_0 \). So the fracture energy is inversely proportional to the slope, \( A \), of the size effect regression line. Note that Eq. 6 does not depend on the hypothesis that the R-curve is unique.

Substituting \( A \) and \( g(\alpha_0) = 43.76 \) from function \( g(\alpha) \), Eq. 6 yields \( G_f = 0.291 \text{ lb/in.} \). This may be compared to the value obtained by Jenq and Shah (15) from their measurements not involving the size effect; it was 0.42 lb/in. This is not too close to our value, however, their method of measurement lends their \( G_f \) value a somewhat different physical meaning.

Evaluating \( Y = AX + C \) for various values of \( d \), we may further obtain smoothed maximum load data \( P \):
\begin{align*}
d & = 1.5, \quad 3, \quad 6, \quad 9, \quad 12, \quad 15, \quad 18, \quad 21 \text{ in.;} \\
P & = 83.0, 154.1, 272.7, 370.8, 455.4, 530.6, 598.6, 661.0 \text{ lbs.} \tag{7}
\end{align*}
(this same smoothed data would be obtained even if the measurements included only the three asterisk-marked values in Eq. 5).

Geometric similarity is required only in two dimensions, and thus the specimens could have different thicknesses \( b \). However, the fracture energy is not constant along the front edge of crack, and thus the thickness has some effect on the mean \( G_f \) for the whole thickness. There are two sources of the thickness effect. One is the disturbance of the free surface boundary conditions due to Poisson effect and surface point singularity of elastic solution (8), and another one is the different influence of the aggregate size near the surface and the interior on the microcracking zone size. The former effect is eliminated if \( b/d \) is constant, and the latter one if \( b \) is constant. Thus, no perfect answer exists for the choice of thickness. The condition \( b = \text{constant} \) seems, nevertheless, more important.

The fact that the size effect plot [Fig. 1(a)] possesses an inclined asymptote implies that the R-curve must have a horizontal asymptote, a property previously sometimes regarded as uncertain. The fracture energy, \( G_f \), a term reserved here strictly for the final asymptotic value of \( R \), is uniquely defined by this asymptote. Thus, the value of the fracture energy as defined here must be considered size-independent. Compared to other definitions of \( G_f \), the present one is most relevant for determining the failure loads at different size structures.

If the three-point bent fracture specimen is sufficiently slender, i.e., \( L/d \) is sufficiently large, the failure is governed primarily by the bending moment \( M \) in the notched cross section, and the effect of the shear forces may be neglected. Then it is not necessary for the specimens of different sizes to be geometrically similar. It suffices if the notched cross sections are similar, i.e., have the same ratios \( a_0/d \). Let the reference specimen be characterized by \( d = d_1 \) and \( L = L_1 \). Then the maximum load \( P \) measured for a dissimilar specimen of dimensions \( L_2, d_2 \) such that \( L_2/d_2 \neq L_1/d_1 \), must be transformed to the maximum load \( P^* \) corresponding to span \( L_2^* = L_1 d_2/d_1 \). Equating the bending moments, \( (1/4) P_1 L_1 = (1/4) P_2^* L_2 d_2 \), we obtain
\begin{equation}
P_2^* = P_1 \frac{L_2^*}{L_1} \frac{d_1}{d_2} \tag{8}
\end{equation}
This value, rather than the measured load \( P_2 \), must then be used in the size effect regression plot [Fig. 1(b)].

If the beam is not very slender, a correction for the dependence of \( g(\alpha) \) on \( L/d_0 \) should be introduced. The similar specimen of span \( L_2^* \) and the actual, dissimilar specimen of span \( L_2 \) have the same cross section, and therefore they may be assumed to fail at the same crack length \( c^* \), i.e., at the same \( a \) or the same \( \alpha (\alpha = a/d) \). Thus, the energy release rates \( G \) at maximum load must be the same for both specimens, and so (according to Eq. 3c) \( G = P_2^2 g(\alpha_0)/E b^2 d_2^2 = P_2^2 g^*(\alpha_2)/E b^2 d_2^2 \) where \( \alpha_2 = a_2/d_2 = a_2^*/d_2^* \); this yields
\begin{equation}
P_2^* = P_2 \sqrt{\frac{g(\alpha_0)}{g^*(\alpha_2)}} \tag{9}
\end{equation}
It may be checked numerically that if both \( L_2/d_2 \) and \( L_2^*/d_2 \) are large, Eq. 9 is reduced to Eq. 8.

Eq. 9 has an inconvenient feature in that the crack length \( c \) at failure must be estimated before \( P_2^* \) can be calculated; \( \alpha_2 = (\alpha_0 + c)/d_2 \). The estimate, however, can be improved iteratively. For this purpose one has to solve also the R-curve as described in the next section, and then solve \( \alpha_2 \) from Eq. 4, upon which one may obtain an improved value of \( P_2^* \) from Eq. 9.

**Identification of R-Curve as an Envelope**

Consider again that maximum load \( P \) for a series of geometrically similar specimens has been measured. However, the crack lengths \( c \) cor-
This yields the family of curves in Fig. 3(a). The envelope may now be easily obtained.

Although, from the viewpoint of representation of the original measured data, many different formulas are about equally good (7) (due to large scatter of data), the choice of formulas becomes more limited if one wants to closely match the size effect law (Eq. 1). From this viewpoint, the following formula appears to work well

\[ R(c) = G_f \left[ 1 - \left( \frac{c}{c_m} \right)^n \right] \quad \text{for} \quad c \leq c_m; \quad R(c) = G_f \quad \text{for} \quad c \geq c_m \quad \cdots \quad (10) \]

According to the values in Eq. 7 \( G_f = 0.291 \) lb/in., \( c_m = 1.895 \) in., and \( n = 3.6 \). As seen from Fig. 3(a), this formula appears to give a nearly perfect envelope. Alternatively, one can also use the formula

\[ R(c) = G_f \left( 1 + \frac{k_0}{c + k_1 c^n} \right)^{-n} \quad \cdots \quad (11) \]

with coefficients \( G_f, k_0, k_1, m, \) and \( n \). It so happens that the approximations of the envelope remain quite good if \( m \) and \( n \) are fixed as \( m = 0.19 \) and \( n = 4.24 \). Then Eq. 11 has the advantage that parameters \( k_0 \) and \( k_1 \) may be determined by linear regression. Indeed, Eq. 11 can be transformed to the form \( y = A'x + C' \), in which \( x = c^{-n} \); \( y = [(G_f R^{-1})/m - 1]^{-1}/c \); \( A' = k_0 C' \); \( C' = 1/k_1 \). For the data in Eq. 7, one obtains \( k_0 = 1.53 \times 10^{10} \) and \( k_1 = 3.58 \times 10^{10} \).

For comparison, let us see what we would get if the size effect law (Eq. 1) were unknown. Then, plotting the curves of \( G \) versus \( c \) on the basis of the measured maximum loads (Eq. 5), we would get the family of curves shown in Fig. 3(b). This family has no common envelope, and since the value of \( c \) at failure for each of the curves is unknown, it is not even possible to deduce any R-curve by statistical regression. Thus, knowledge of the size effect law (Eq. 1) is crucial for being able to determine the R-curve without having to measure the crack lengths at failure, a task notorious for its difficulty and ambiguity. For comparison, Fig. 3(b) also shows the R-curve which is obtained if the measured maximum loads are first smoothed with the size effect law, Eq. 1.

Instead of constructing the envelope graphically [Fig. 3(a)], one can define it analytically. For this purpose, we insert \( P = \sigma_f a \) in Eq. 3a, and we set equal to 0 the partial derivative of this equation with respect to \( \lambda \) at constant \( G \), which is the condition for an envelope. This yields

\[ G = \frac{d_0}{E} \lambda \sigma_f^2(\alpha) g(\alpha); \quad \alpha = \frac{d_0 + c}{\lambda d_0} \quad \cdots \quad (12) \]

\[ a_0 + c \quad 1 \quad d_g(\alpha) = \lambda + \frac{2\lambda^2}{\sigma_f(\lambda) \sigma_f(\lambda)} \quad \cdots \quad (13) \]

These equations represent a parametric equation of the R-curve, with \( \lambda \) as a parameter. To calculate points on the R-curve, a series of \( \lambda \) values is chosen, and for each \( \lambda \) the value of \( c \) is solved from Eq. 13 by Newton...
TABLE 1.—Size Effect Curves

<table>
<thead>
<tr>
<th>Depth, ( d )</th>
<th>Smoothed max. ( P ) data:</th>
<th>After conversion to R-curve and back:</th>
<th>After regression of Col. 2:</th>
</tr>
</thead>
<tbody>
<tr>
<td>(1)</td>
<td>( B_f = 60.39, d_0 = 7.834 )</td>
<td>( c_e = 1.895, n = 3.6 )</td>
<td>( B_f' = 59.59, d_0 = 8.180 )</td>
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<tr>
<td>0.1</td>
<td>60.01</td>
<td>59.05</td>
<td>59.23</td>
</tr>
<tr>
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</tr>
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<tr>
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<td>7.50</td>
<td>7.49</td>
<td>7.56</td>
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</table>

specimen geometry may be considered as approximately unique.

In the size effect law (Eq. 1), there are two independent parameters, \( B_f' \) and \( d_0 \). For the relative R-curve values \( R(c)/G_i \) in Eq. 10, there are also only two parameters, \( n \) and \( c_e \) (\( G_i \) is obtained by linear regression from Eq. 10). Hence, for all specimens for which the values of \( B_f' \) and \( d_0 \) are the same, the values of nondimensional parameters \( n \) and \( c_e \) should

TABLE 2.—Values of \( R(c)/G_i \) for Curves in Fig. 4(b) Defining Relative Shapes of R-Curves for Different Specimen Types and the Same Size Effect Law (Eq. 1)

<table>
<thead>
<tr>
<th></th>
<th>Three-Point Bent</th>
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<th></th>
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<td></td>
<td>( l/d = 4; )</td>
<td>( l/d = 4; )</td>
<td>( l/d = 8; )</td>
<td>( \text{Compact tension}; )</td>
</tr>
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<td></td>
<td>( a_i/d = 1/3, )</td>
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<tr>
<td></td>
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<td>( n = 3.6, )</td>
<td>( n = 3.6, )</td>
<td>( n = 4.0, )</td>
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<td></td>
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<td>( c_e = 1.491, )</td>
<td>( c_e = 1.819, )</td>
<td>( c_e = 2.280, )</td>
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<td>( c_{\text{th}} = 0.332 )</td>
<td>( c_{\text{th}} = 0.261 )</td>
<td>( c_{\text{th}} = 0.336 )</td>
<td>( c_{\text{th}} = 0.363 )</td>
</tr>
<tr>
<td>( c/c_{\text{th}} )</td>
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</table>
FIG. 5.—Parameters of R-Curve Corresponding to Size Effect Law

also be the same. This means that all geometrically similar specimens of different sizes should be characterized by the same values of \( n \) and \( c_m/d_0 \). This property is verified by numerical examples. We may, therefore, construct a table of \( n \) and \( c_m/d_0 \) for various typical specimen geometries, see Fig. 5. For the specimen geometries in Fig. 5, it is not necessary to construct the R-curve as an envelope. It suffices to carry out the linear regression shown in Fig. 1(b), and then take \( n \) and \( c_m/d_0 \) from Fig. 5.

For comparison, consider now a series of tests of specimens of the same dimension \( d \) but different notch lengths \( d_0 \). The maximum loads \( P \) are measured for various \( d_0 \). If one wishes to use the envelope property, one must plot for each pair of \( d_0 \) and \( P \) the curve of \( G \) versus \( c \)

where \( G = \frac{P^2}{(E_r b^2 d)} \) and \( \alpha = \frac{a_0 + c}{d} \). If the measurements were perfect, then these plots would yield a family of curves such as illustrated in Fig. 6(a), for which the envelope representing the R-curve can be, in theory, constructed. In practice, this does not work, for two reasons: (1) If \( d \) is constant, only a small portion of the R-curve is covered by the failure states for various \( a_0 \) values [Fig. 6(a)]. There is always statistical scatter, which causes that these plots yield a family of curves such as illustrated in Fig. 6(b). Obviously, no envelope can be constructed, and thus smoothing of the data is imperative before the envelope could be traced. However, for the effect of the notch length \( a_0 \) at a constant cross section dimension \( d \), there is no simple law which could be used for smoothing the data. A considerably more sophisticated procedure is required to determine the material parameters (7).

Determining \( G \) from Complete Load-Deflection Diagram.—It has been suggested that the fracture energy \( G \) can be determined as the area under the complete load-deflection diagram (17,18,20). However, the \( G \) values obtained in this manner are frequently inconsistent and scattered. One source of error is that on larger specimens, due to the specimen's own weight, the declining load-deflection diagram cannot be measured down to zero due to instability, and the tail must be estimated. Another source of error is that energy dissipation which does not produce fracture may occur in the system.

Still another source of error and a basic difference from the present approach is that the load values for all crack lengths affect the result, while here only the peak load value matters. The question then is whether the fracture process zone at other than peak load values dissipates energy at the same rate as it does at the peak load. This would certainly be true if the fracture were a straight line, with a sharp tip and a fracture process zone of negligible size. This is not so, however. It is likely that the width and the length of the microcracking zone at the fracture front are different than they are at the peak load, and the fact that \( G \) is far larger than double the theoretical specific surface Gibbs' free energy of the solid serves as the proof that much energy dissipation must occur.

FIG. 6.—R-Curve as Envelope for Various Notch Lengths in Specimen of One Size: (a) Smoothed; (b) Unsmoothed Data

FIG. 7.—(a) Uniaxial Stress-Strain Relation for Crack Band Model; (b) Finite Element for Crack Band Model; (c) \( \sigma \)-\( \delta \) Relation for fictitious Crack Model of Hillerborg; (d–h) Explanation of Energy Dissipation during Crack Growth
due to microcracking on the sides of the final continuous crack. The width of the microcracking zone ahead of the fracture front is assumed to be constant in the crack band model as well as Hillerborg's fictitious crack model, but this is no doubt a simplification. In fact, it is likely that the width \( w \) of this zone varies [Fig. 7(g)], and especially that the widths at crack initiation [Fig. 7(h)] and at crack termination [Fig. 7(f)] may be quite different from the width at the peak load [Fig. 7(g)], at which the fracture front is remote from both the notch and the opposite face. Consequently, \( G \) along the crack path is variable [Fig. 7(h)] and the mean value \( G_0 \) (obtained by dividing the area under the load-deflection curve by the ligament length \( d - a_0 \)) need not be the same as the value of \( G_f \) at maximum load [Fig. 7(h)]).

Now, of the values \( G \) and \( G_f \) [Fig. 7(h)], which one is more useful? That depends. If we need to predict the peak loads or the response near the peak loads, then it is more reasonable to use only peak load values for determining \( G_f \). Besides, they are easier to measure.

In light of Fig. 6, there seems to be still another source of error. From this figure it is apparent that the use of a single size specimen with different crack lengths cannot, due to inevitable random scatter, give information on the complete R-curve, or the complete size-effect curve. It pertains only to a portion of the curve, and does not indicate unambiguously the limiting value of the R-curve, which represents \( G_f \) (Fig. 6).

If tests on single size specimens (without crack length measurements) do not give sufficient data on the R-curve, how can they unambiguously yield the R-curve asymptote?

R-Curves for Different Specimen Shapes.—The crack band model makes it possible to illustrate how much error is introduced when the R-curve determined for one specimen shape is used for another specimen shape. Calculations of R-curves for different geometries have been made using the same crack band parameters, see Fig. 8. The differences between some of these curves may seem large, however, they are not large compared to the inevitable statistical scatter of measured \( R \)-curve values. Therefore, the hypothesis of a unique \( R \)-curve appears to be acceptable for crude structural analysis, within a certain range of geometries. There exist, though, geometries for which a substantially different \( R \)-curve must be used (e.g., double-edge-cracked strip in tension versus eccentric compression).

**Identification of Crack Band Model Parameters**

The size effect law (Eq. 1) may be also exploited to identify the material parameters for the finite element crack band model (3,10). In this model the fracture energy is equal to the area under the tensile uniaxial stress-strain diagram in Fig. 7(a), times the effective width of the cracking front, \( w \). Assuming this diagram to be bilinear, with elastic modulus \( E \) and mean strain-softening modulus \( E_s \) (\( E_s \) = 0), we have \( G_0 = f_t^2 (1 - E_s / E) w / 2E \) from Ref. 9. Approximately, \( w = 3d \), where \( d \) is maximum aggregate size (10).

For geometrically similar specimens, the finite element solution of the nominal stress at failure \( \sigma_{f_0} \) should depend only on the material parameters. There are four of them: \( f_t^0 \), \( E \), \( G_f \), and \( w_0 \). Parameter \( E \) is not independent since it is related to \( G_f \). Consequently, for geometrically similar specimens, \( \sigma_{f_0} = \frac{\sigma_{f_0}}{w_0} = \frac{E f_t^0}{w_0} \) in which \( \phi \) is a certain function. Now, according to Buckingham’s II-theorem of dimensional analysis (4), the number \( n \) of independent nondimensional governing parameters should be \( n = 4 - 2 \) where \( n = 4 \) number of governing parameters and \( n = 2 \) number of independent dimensions in these parameters (length and force). Thus, \( n = 4 - 2 = 2 \). So there can be only two independent nondimensional governing parameters. Along with the nondimensionalized function \( \sigma_{f_0} \), they may be introduced as

\[
s = \frac{\sigma_{f_0}}{f_t^0} ; \quad \theta = \frac{d}{w_0} ; \quad \kappa = \frac{E G_f}{w_0 f_t^0} \quad \text{...........................................(14)}
\]

and function \( \phi \) reduces to

\[
s = \phi(\theta, \kappa) \quad \text{...........................................(15)}
\]

Eqs. 14–15, describing the similitude of blunt fracture, greatly reduce the number of cases that have to be solved by finite elements in order to cover all possible situations.

For identifying the material of the crack band model, the following approach may be adopted. We choose a certain specimen geometry, and by testing specimens of different sizes we determine, by linear regression, parameters \( B \) and \( d_0 \) of the size effect law (Eq. 1). Then, by carrying out finite element solutions for geometrically similar specimens of different sizes, we determine the size effect plot as a function of the governing parameters \( \theta \) and \( \kappa \) (Eq. 15). Finally, we determine the values of \( B \) and \( d_0 \) for which Eq. 1 gives the optimum fit of this plot. The interception of the size effect law not only facilitates analysis, but also has the advantage of smoothing and extending randomly scattered measured data. The detailed procedure is as follows:

**Fig. 8.—R-Curves Calculated by Crack Band Model for Same Material Parameters and Different Specimens**
Nonlinear finite element solution is not needed if equivalent linear fracture analysis is used. The value of $G_f$ may then be calculated from Eq. 6 on the basis of the slope of the size effect regression plot and the value of $g(a_0)$ obtained by an analytical or finite element solution according to linear elastic fracture mechanics.

In the preceding procedure based on Eq. 16, the value of $w_0$ has been considered as unknown. As a rough approximation, however, $w_0 \approx 3d_i$ where $d_i =$ maximum aggregate size (9). If this approximation is adopted, and if $f_i$ is known, then $G_f$ is simply obtained from Eq. 16 on the basis of the slope of the regression plot for the size effect [Fig. 1(b)], and $E_i$ is then evaluated from Eq. 16.

**IDENTIFICATION OF NONLINEAR FRACURE MODELS WITH STRESS-DISPLACEMENT RELATION**

Through a simple extension of the foregoing analysis, it is possible to determine the material parameters for Hilleborg's (12,17) and other nonlinear fracture models, in which a sharp crack is assumed and a relation between the relative displacement $\delta$ and normal stress $\sigma$ across the line is introduced as a material property. The displacement $\delta$ jumps into a line the normal strain due to cracking accumulated over length $w_0$, and so

$$\delta(\sigma) = w_0 \left[ \frac{\epsilon(\sigma) - \sigma}{E} \right] \quad (17)$$

where $\epsilon(\sigma)$ describes the tensile stress-strain diagram for the equivalent crack band model. Most simply, the $\sigma-\delta$ relation may be considered as a straight line of negative slope $C_f$ [Fig. 7(c)], described as $\delta(\sigma) = (f_i - \sigma)/C_f$, if $\sigma \geq 0$ and $\delta \geq 0$, but $\sigma = 0$ if $\delta \geq f_i/C_f$. Equating the total relative displacement across the crack band according to both models and assuming the stress at the crack front (in a thin plate) to be approximately uniaxial, we have the relation $w_0 \sigma/E + (f_i - \sigma)/C_f = w_i[f_i/E - (f_i - \sigma)/E_i]$. This relation is satisfied for any $\sigma$ if

$$\frac{1}{C_f} = w_i \left( \frac{1}{E} \frac{1}{E_i} \right) \quad (18)$$

Alternatively, we may use the condition of equal fracture energy for both theories; $G_f = f_i^2/2C_f = w_i(E^{-1} - E_i^{-1})f_i^2/2$. From this, Eq. 18 results again.

**USE OF GENERALIZED SIZE EFFECT LAW**

Eq. 1 is the simplest possible approximation to the size effect for blunt fracture. The most general size effect law may be stated in the form of the asymptotic expansion: $\sigma = B f_i (a_0 \eta^{-1} + 1 + a_1 \eta + a_2 \eta^2 + a_3 \eta^3 + \ldots)^{-1/2}$ where $\eta = (d/d_i)^{f_i^2}$ and $f_i$, $B$, $r$, $a_0$, $a_1$, $a_2$, $a_3$, $\ldots$ are material parameters (6). The first order approximation obtained by setting $a_1 = a_2 = a_3 = \ldots = 0$, i.e.,

$$\sigma = B f_i \left[ 1 + \left( \frac{d}{d_i} \right)^{-1/2} \right]^{-1/2} \quad (19)$$

![Table](image)

**FIG. 9.—Coefficients for Determining Crack Band Model Parameters**

1. Set $w_0 = E = f_i^2 = b = 1$, and find the value of $\kappa$ ($\kappa = G_f$).
2. Fix the value of $\theta$ ($\theta = d$) and solve by a finite element program with incremental loading the maximum load $P$ (the load-point displacements are prescribed and $P$ is calculated as the reaction). Then calculate $s = P/\theta$.
3. Repeat step 2 for various $\theta$ values and construct the plot of $s$ versus $\theta$.
4. Now, according to the size effect law in Eq. 1, this plot should ideally agree with $s = B [1 + (\theta/d)]^{-1/2}$, which is equivalent to $y = Ax + C$ where $x = \theta; y = s^{-1/2}; A = (B^2 r)^{-1}; C = B^{-2};$ and $r = d_0/w_0$. Determine the regression line of this plot; its $y$-intercept is $C$, from which $B = 1/\sqrt{C}$, and its slope is $A$, from which $r = C/A$. These values of $B$ and $r$ correspond to the previously fixed value of $\kappa$.
5. Repeat steps 1–4 for various values of $\kappa$ and construct the graphs $B(\kappa)$ and $r(\kappa)$.

The numerical results show that the graphs of $B(\kappa)$ and $r(\kappa)$ are linear. Thus, calculation of only two points on each graph is sufficient, and the values may generally be expressed as $\kappa = k_1 + k_2 B$, and $r = k_3 + k_4 B$. Fig. 9 shows the calculation results for several typical fracture specimens.

As an example, consider again the data from Eq. 7. The test results are plotted as $1/\sigma_i^2$ versus $d$ [Fig. 1(b)], and from the slope and the intercept of the regression line we get the values of $B$ and $d_0$ (Eq. 1) as measured. For the data in Fig. 1(b) we have $B = 0.1817$ and $d_0 = 7.834$. Then, for this value of $B$, and using the values from Fig. 9, we get $\kappa = 11.9$ and $r = 8.4$. From this we finally obtain

$$w_i = \frac{d_0}{r} ; \quad G_f = \frac{w_i}{E} f_i^2 \kappa ; \quad E_i = \left( \frac{1}{E} - \frac{2G_f}{w_i f_i^2} \right) \quad (16)$$

Exploiting the size effect law (Eq. 1) makes it possible to do with a lesser amount of measurements. If the maximum load values for only a few specimens are fitted directly with the finite element program for the crack band theory, the values of material parameters which give good fits of data are quite ambiguous; even very different material parameter values yield equally good fits. This ambiguity and uncertainty is removed by the size effect law, which has the effect of smoothing and extrapolating the measured data.
is considerably more flexible than Eq. 1 for describing various possible shapes of the curve of log $\sigma$ versus log $d$ which may be obtained by finite element calculations (2). However, the size range of the available test data does not suffice for unambiguous determination of $r$.

If sufficiently broad data were available, then Eq. 19 could also be used to determine fracture parameters. Eq. 19 may be written as $Y = AX + C$ in which $X = d^{-r}$, $Y = a/d$, $A = C(\alpha_a d_a)^{r-2}$ and $C = (B f)^{3-r}$. A set of $r$-values, e.g., 1, 0.8, 0.6, 0.4, may be chosen and, for each, one calculates by linear regression the values of $A, C$ and the sum of squared deviations from the test data. One considers the dependence of this sum on $r$ and identifies the $r$-value that minimizes this sum. Then $B = C^{1/r} f_i / \alpha_a = (CA)^{1/r} / d_i$. For $d_i / d_a \rightarrow \infty$, we have the relation $a/d = B f_i^{3-r} d / d = G f_i / (\alpha_a d)$, from which we get for $G$ again Eq. 6. The $G$ values obtained for various $r$ are rather different; however, within a not too broad range of sizes for which the tests were made, the predictions of fracture behavior are quite close to each other.

**Conclusions**

1. The size effect law of blunt fracture (Eq. 1) is useful for identifying the material parameters for nonlinear fracture, regardless of whether the R-curves approaches, or the strain-softening crack band model, or the stress-displacement relation (Hillerborg’s model) is used. The basic idea is to transform the size effect law to a linear plot and determine in this plot the regression line for the measured data obtained by tests of geometrically similar specimens of different sizes. The slope of this regression line then yields the fracture energy (the value of which is, by definition, size-independent), and the $y$-intercept yields the strain-softening modulus. The method can also be extended to certain dissimilar specimens of similar cross sections. The size effect law, the parameters of the crack band model, and the R-curves parameters are uniquely related. If one of them is specified, the others follow.

2. Exploiting the size effect law has four advantages:
   a. Statistically scattered measurements are smoothed [Fig. 1(b)].
   b. The range of the test data is extended, thus reducing ambiguity of data fitting and uncertainty in the material parameter values. Consequently, the experimentalist can get by with fewer tests covering a narrower range.
   c. Determination of R-curves as an envelope is made possible, while without the smoothing by the size effect law no envelope exists.
   d. A simpler measurement procedure than with the existing methods becomes possible. Only maximum load values are needed. They can be obtained even in a laboratory with the most rudimentary equipment. There is need to measure the crack length, which avoids the ambiguity in defining and observing the location of the crack tip. No measurement of unloading or re-loading compliance is needed.

3. When nonlinear fracture mechanics is applied for the purpose of determining the maximum load for monotonic loading, rather than the maximum load after a series of previous unloadings, it is more realistic to use only maximum loads also for the experimental calibration of the mathematical model.

4. Measuring maximum loads of specimens that have notches of various lengths but are of the same size does not provide sufficient basis for determining the nonlinear fracture properties, e.g., the R-curve.

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**APPENDIX.—REFERENCES**


