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ABSTRACT

Presented is a particle model for brittle aggregate composite materials such as concretes, rocks or ceramics. The model is also applicable to the behavior of unidirectionally reinforced fiber composites in the transverse plane. A method of random computer generation of the particle system meeting the prescribed particle size distribution is presented. The particles are assumed to be elastic and have only axial interactions, as in a truss. The interparticle contact layers of the matrix are described by a softening stress-strain relation corresponding to a prescribed microscopic interparticle fracture energy. Both two- and three-dimensional versions of the model are easy to program, but the latter poses, at present, forbidding demands for computer time. The model is shown to realistically simulate the spread of cracking and its localization. Furthermore, the model exhibits a size effect on: (1) the nominal strength, agreeing with the previously proposed size effect law, and (2) the slope of the post-peak load-deflection diagrams of specimens of different sizes. For direct tensile specimens, the model predicts development of asymmetric response after the peak load.

INTRODUCTION

It is now generally accepted that inelastic deformation and fracture of brittle heterogeneous materials such as concrete or fiber composites can be adequately modeled neither by classical (local) continuum material models nor by linear elastic fracture mechanics. Although nonlinear fracture models and nonlocal continuum models can do a long way towards a realistic description, they are inherently incapable of capturing those phenomena which are strongly affected by the randomness of material heterogeneity on the microscale and are localized to a small but non-negligible region. The continuum models can describe well the mean of the macroscopic material response but not its variance. Even when generalizations such as the stochastic finite element method are considered, the assumed spatial randomness can only be correct in the overall sense and cannot capture the effect of random local material inhomogeneities on the localization of damage and failure. It is for these reasons that a direct simulation of the random microstructure of these materials is useful. As demonstrated by Zubelewicz and Bažant (1987), a model simulating the microstructure can describe progressive...
The present paper summarizes the results of a study which will be reported in detail elsewhere (Bazant et al., 1990). The principal objective of this study has been to determine the effect of the size of the specimen or structure on the maximum load, the subsequent softening behavior, and the spread of the microcracking zone. These are the key questions of fracture modeling. The material will be modeled as a system of randomly arranged circular particles. This model is intended to simulate the behavior of concrete, but can also be used for the behavior of unidirectionally reinforced fiber composites in the transverse plane, with microcracks parallel to the fibers and spreading in the transverse direction.

The present model is a modification and refinement of that recently developed by Zubelewicz and Bazant (1987). The idea of particle simulation is an older one; it was proposed by Cundall (1971), Serrano and Rodriguez-Ortiz (1973) and Kawai (1980). These models, which dealt with rigid particles which interact by friction and simulate the behavior of granular solids such as sand, were developed and extensively applied by Cundall (1978) and Cundall and Strack (1979), who called it the distinct element method. An extension of Cundall's method to the study of microstructure and crack growth in geomaterials with finite interfacial tensile strength was introduced by Zubelewicz and Mróz (1983), and Plesha and Alfantis (1983).

In the present model we neglect the shear and bending interaction of neighboring particles throughout their contact layers. With such a simplification the present model becomes similar to the random truss model of Burt and Dougill (1977). These investigators verified that the truss model yields a realistic strain-softening curve but they studied neither the fracture mechanics nor size effect aspects, and their method of system generation was not based on the idea of particles with a prescribed gradation.

SUMMARY OF MATHEMATICAL FORMULATION

The adjacent circular or spherical particles shown in Fig. 1 are considered to interact only on the axial direction. Each strut connecting two adjacent particles consists of three segments simulating the aggregate particle and the interparticle contact layer. The two end segments, representing the particles, are assumed to be always elastic, with modulus $E_a$.

The central segment of the strut, representing the interparticle region of the matrix, is assumed to exhibit softening characterized by the triangular stress-strain relation shown in Fig. 2. The softening segment of the stress-strain relation varies so as to give the same stress-displacement relation regardless of the length of the central segment of the strut, thus assuring the fracture energy of this strut segment, represented by the area under the stress-displacement curve, to be constant.

A simple method to generate the random particle system has been developed. The generation begins with the particles of the largest size, their coordinates are generated with a random number generator assuming a uniform probability density throughout the specimen area. For each generated location of the particle center, the program checks for possible overlaps with already generated particles, and the location is rejected if an overlap occurs. The random generation of particle locations then proceeds to the next smaller size, and the same procedure is repeated. The program continues until the last particle of the smallest size has been placed within the specimen.

The basic idea that makes the foregoing generation of the random particle system possible is that the particles are assumed to interact not only when they are in contact, but also when their influence regions overlap. These influence regions are assumed to be circles or spheres of a diameter which is $\beta$-times larger than the diameter of the particle (Fig. 1), $\beta$ being an empirical constant ($\beta=1.65$ has been used).

RESPONSE OF UNHOTCHED SPECIMENS

Geometrically similar specimens (Fig. 3) with depths $= 3, 6$ and $12$ times the maximum particle size have been analyzed. The length of the specimen is always $L = 8d/3$. The diameters of the particles are $12, 8, 5, 3$ and $1$ mm, the volume ratio for each diameter is selected so as to model concrete. The response of the particle system can be solved by the same methods as used in nonlinear finite element analysis. The technique of incremental loading with iteration in each loading step has been applied to the equivalent truss mesh for each specimen. Fig. 4 shows such a mesh for a typical specimen.

Size Effect

A salient consequence of heterogeneity, which has fracture mechanics as well as probabilistic aspects, is the size effect on the failure load. If the failure is governed by criteria in terms of stress or strain (yield criteria), then geometrically similar specimens of different sizes must fail at the same value of the nominal stress at failure $\sigma_u$ defined as $P/A$, where
Fig. 3 Geometrically similar specimens of various sizes with randomly generated particles.

To test this property, the specimens shown in Fig. 3 were subjected to prescribed uniform longitudinal displacement \( u \) at one end and restrained against displacements at the opposite end (Fig. 5). Displacement \( u \) has been incremented in small steps, and the axial force resultant \( P \) was calculated for the following material properties: \( G_f = 24 \text{ N/m} \), \( f_t = 3 \text{ MPa} \), \( E_s = 30 \text{ GPa} \), and \( E_s = 6E_s \). The diagrams of load \( P \) versus load-point displacement \( u \) have been constructed; they are plotted in Fig. 6 for the small, medium and large specimens. Also shown are the load-deflection curves for specimens 1A, 2A and 3A scaled so that the peak point would coincide with \((1,1)\). These curves reveal that the post-peak declining slope gets steeper as the specimen size increases.

The data points for maximum loads are plotted in Fig. 7 in terms of stress \( \sigma_u \) as a function of the specimen depth \( d \). The nominal stress is normalized with respect to \( f_u \), an arbitrary measure of material strength, taken here as \( f_u = f_t = 3 \text{ MPa} \). The depth is normalized with respect to \( d = \text{diameter of the largest particle} = 12 \text{ mm} \). We see considerable scatter; therefore, the values of the nominal stress are averaged for each specimen size and are plotted as asterisks. These plots clearly reveal that there is a size effect.

Fig. 4 A typical randomly generated specimen and its corresponding mesh of truss elements.

Fig. 5 Direct tension specimens with \( d = 36, 72 \) and \( 144 \text{ mm} \)
The size effect for materials that exhibit progressive cracking is known to approximately follow the size effect equation proposed by Bążant (1984):

\[ \sigma_u = \frac{Bf_u}{1 + d/d_0} \]

where \( d_0 \) and \( B, \lambda_0 \) are empirical parameters. Eq. 1, which is approximate but usually applicable to size ranges up to 1:20, represents a gradual transition between the strength (or yield) criterion, for which there is no size effect in nominal strength, and linear elastic fracture mechanics, for which the size effect is the strongest possible (\( \sigma_u = d^{-1/2} \)). For data fitting it is convenient to transform Eq. 1 to the linear regression plot \( Y = AX + C \), in which \( X = d/d_0, Y = (f_u/\sigma_u)^2, C = 1/B^2 \), and \( A = C/\lambda_0 \).

For comparison, the lines representing the size effect according to the strength (or yield) criterion and according to the linear elastic fracture mechanics are also shown in Fig. 7b. From this it is evident that the size effect obtained is intermediate between the strength criterion and the linear elastic fracture mechanics. This may be expected on the basis of the fact that the specimens do not fail at cracking initiation but only after a crack band has already developed, in which case nonlinear fracture mechanics should be applicable.

Progressive Spread of Cracking

Fig. 8 shows for various specimens their microcrack patterns at the last calculated point on the load-deflection curve. Fig. 9 shows these patterns as
Fig. 8 Cracking patterns at the last calculated point on the load-displacement curve for unnotched specimens in tension.

Fig. 9 Evolution of cracking with load level for specimen 1A.
they develop in specimen 1A at various stages of loading corresponding to points 1, 2, 3 and 4. The fully formed and open cracks are shown by the solid lines, and the partially formed, developing (active) cracks which correspond to strain-softening states are shown by the dashed lines. Also shown are the previously formed cracks which are getting unloaded; these are represented by asterisks. We see that the cracks first start at many random locations throughout the specimen, but later many of these partially formed cracks unload, and only some of them, lying in a narrow transverse band, open further and lead to the final fracture.

Scatter of Stress Profile and Symmetry Breakdown

The distribution of the longitudinal component of the interparticle forces $P_i$ throughout the cross section at distance $x = 60$ mm from the fixed end is sketched, for specimen 1A, at various stages of loading in Fig. 10. We see that the nonuniformity in the distribution is getting more pronounced at later stages of loading. Furthermore, we see that the resultant of the longitudinal force tends to shift away from the center line, which indicates a tendency for these specimens to follow an asymmetric deformation pattern. The eccentricity, $e$, of the force resultant, $P$, is plotted as a function of $P$ for two specimens (1A and 1B) in Fig. 11, in which the tendency towards an increasing eccentricity is clearly seen. A similar asymmetry was reported by Rots, Hoek, and de Borst (1987) who studied by finite elements the evolution of crack bands in concrete tensile specimens.

Macroscopic Poisson's Ratio

To determine the Young's elastic modulus $E$ and the macroscopic Poisson's ratio of the equivalent (smearred, homogenized) elastic continuum, the large specimen 3A was loaded in uniaxial tension by prescribing a very small uniform longitudinal displacement ($u = 0.004$ mm) at one end and restraining the specimen against displacement at the other end. Free sliding was allowed at the ends in the transverse direction. The displacements in the longitudinal direction, $u_x$, and the transverse direction, $u_y$, at the nodes with the maximum size aggregates were fitted by a linear field to determine the mean axial and lateral strains. To minimize the effect of boundary conditions, only nodes within an interior region ($0.8d$ x $0.8L$) were considered. For the mean uniaxial stress $\sigma_x = 0.39$ MPa, the slope of the linear regression line for $u_y$ as a function of the $x$-coordinate, which represents the mean (macroscopic) strain.

Fig. 11 Evolution of eccentricity as a function of load resultant.

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Fig. 11 Evolution of eccentricity as a function of load resultant.
in the x-direction, indicated the value \( e = 0.1154 \times 10^{-4} \). From this, 
\[ E = 3.4 \times 10^9 \text{ MPa}. \]
A similar regression line gave the strain in the y-direction, 
\[ e_y = -0.4234 \times 10^{-5}. \]
The macroscopic Poisson ratio was then obtained as 
\[ \nu = -\frac{e_y}{e_x} = 0.37. \]
This value is close to the value \( \nu = 1/3 \), which is the theoretical value for a very large two-dimensional random lattice that is statistically uniform and isotropic. Neither the calculated value \( \nu = 0.37 \) nor the theoretical value \( \nu = 1/3 \) for very large specimens is realistic for most particulate composites, including concrete, for which, typically, \( \nu = 0.18. \)
The correct Poisson ratio of real particulate composites cannot be attained with the present model. The reason for this limitation is that only axial interactions between the particle centers are taken into account while shear stresses in the contact zones are neglected.

**RESPONSE OF NOTCHED SPECIMENS**

As we have seen, cracking in unnotched specimens that are initially stressed uniformly develops quite randomly. This prevents it to bring to light fracture properties of the particle system. To see these properties, notched specimens have also been simulated with the particle model. The notches were mathematically modeled by first generating the random particle configuration as if no notch existed, and then severing all the interparticle connections that cross the line of the mathematical notch depth, \( a_0 = d/6 \).

Fracture properties may be best determined by studying the size effect, since this effect represents the most important consequence of fracture mechanics. To this end, we analyze geometrically similar three-point-bend fracture specimens of three different sizes whose ratios are 1:2:4. Thus, three new specimens were generated as shown in Fig. 12.

In the calculations of maximum loads, three different sets of matrix properties have been considered: (1) \( G^m = 24 \text{ N/m}, f^m = 3 \text{ MPa}, \) (2) \( 24 \text{ N/m}, 1.5 \text{ MPa} \) and (3) \( 240 \text{ N/m}, 3 \text{ MPa} \) with \( E^m = 30 \text{ GPa} \) and \( E^m = 6E \) for all cases. For each set, the regression line (plotted in Fig. 13a) yields the values of slope A and intercept C, from which the values of B and \( \lambda_0 \) follow; see Fig. 13b. The size effect curve according to Eq. 1 is also plotted, along with all the calculated points for the different sets of matrix properties. The results show a good agreement with Eq. 1. However, for the aforementioned values of material properties, the results are closer to linear elastic fracture mechanics than the test results obtained for similar specimens by Bažant and Pfeiffer (1987).

From the size effect on the maximum loads, one can determine the fracture energy \( G_f \) of the idealized material represented by the random particle system, using the formula (Bažant and Pfeiffer, 1987)
\[ G_f = \frac{g(a)}{AE} \frac{B^2}{d} \]
Here \( E = \text{Young's modulus (macroscopic)}, \) and \( g(a) \) is the nondimensional energy...
release rate according to linear elastic fracture mechanics; \( g'(e) = 6.07 \) for the present specimens. One can also determine the size of the fracture process zone according to the formula (Bažant and Kazemi, 1988):

\[
\frac{g'(e)}{g'(0)} = \frac{A_e}{g'(0)}
\]

where \( g'(e) \) is the derivative of \( g(e) \) which is evaluated at \( e_0 = 1/6 \), and is equal to 35.2 for the present three-point-bend specimens.

Table 1 gives the values of \( G_f \) and \( c_f \) calculated for the three sets of matrix properties. Taking the properties in column (1) as reference, the results from column (2) show that a decrease of interparticle strength \( f_0 \) will decrease \( G_f \) and increase \( c_f \). However, the results from column (3) show that an increase of \( G_f \) will increase both \( G_f \) and \( c_f \). Table 1 also includes the values obtained by Bažant and Kazemi (1988) from experiments on mortar and concrete. These values are in the same range as calculated here and could be matched even closer by adjusting the material properties of the model.

### Table 1 Values of Fracture Energy \( G_f \) and Process Zone Size \( c_f \)

<table>
<thead>
<tr>
<th>Three Point Bending</th>
<th>Particle Model</th>
<th>Experiment</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>(1)</td>
<td>(2)</td>
</tr>
<tr>
<td>( G_f ) (N/m)</td>
<td>16.5</td>
<td>6.8</td>
</tr>
<tr>
<td>( c_f ) (mm)</td>
<td>3.5</td>
<td>4.2</td>
</tr>
<tr>
<td></td>
<td>20</td>
<td>37</td>
</tr>
<tr>
<td></td>
<td>1.9</td>
<td>13.5</td>
</tr>
</tbody>
</table>

**CONCLUSIONS**

1. Random generation of a particle system with a given particle size distribution can be accomplished quite easily, due to the fact that the particles are circular (or spherical) and are not in contact but are separated by contact layers whose thickness is unspecified.

2. Modeling only the axial interactions of neighboring particles seems sufficient to obtain a realistic picture of the spread of cracking and fracture in concrete. The neglect of shear interactions might cause the fracture process zone to be shorter than the value reported from test on concrete specimens. However, the length of this zone can be increased by decreasing the value of the interparticle fracture energy \( (G_f) \) or by increasing the value of the interparticle strength \( (f_0) \).

3. The model can describe realistically the post-peak declining load-deflection diagram, which is predicted to get steeper as the specimen size increases.

4. In contrast to local continuum models, the present model is capable of describing the size effect on the nominal strength of unnotched specimens as well as notched specimens.

5. For direct tensile specimens, the model predicts development of asymmetric response after the peak load, such that the specimen bends and the axial force resultant becomes eccentric.

6. Some improvements of the model might be appropriate in: (1) determining the cross section areas of the truss members so as to make them dependent on the distance between particles, (2) controlling the Poisson's ratio without having to introduce shear interaction, (3) developing a method to connect the particle system in the cracking zone to a finite element mesh around this zone. This would significantly reduce the number of particles needed to analyze three-dimensional or large two-dimensional problems.

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